



DEVELOPMENT OF A HYDROLOGIC MODELING SYSTEM FOR THE DRY VALLEY CATCHMENT, PUTNAM COUNTY, TENNESSEE John T. Brackins¹, Alex J. Davis¹, Sarah E. Wilson¹, and Alfred J. Kalyanapu, Ph.D.¹ TECH ¹Department of Civil and Environmental Engineering - Tennessee Technological University

Introduction

In July 2015, the karstic Dry Valley area southeast of Cookeville experienced massive flash flooding due to lack of drainage combined with a greater than 50-year rainfall event. This flood resulted in the severe damage of several properties including a used car dealership. A HEC-HMS "To **explore** the karst drainage flooding problem in the Dry Valley area and develop a HEC-HMS model to simulate the 10-year, 50year, 100-year and 500-year floods for this region while also analyzing previous storm events."



Figure 1a: Dry Valley Watershed south of I-40 through Cookeville Figures 1b-1e: Dry Valley flooding propensity (radar image courtesy NOAA, flood image courtesy WSMV)

Research Objective

e 1: NOAA Precipitation Frequency Dataset (PFD) Data		
Model	Depth (inches)	
10-year	5.10	
50-year	6.67	
100-year	7.38	
500-year	9.12	
Past Event	6.74	

- Flow lengths calculated within ArcGIS

- hydrographs
- Assumptions:



Data Processing



Figure 3: Subbasin delineation

Watershed divided into 122 subbasins:

Ranged in area from 2.6 acres to 500 acres

 SCS Curve Number assigned based on areal average from NLCD 2011 and SSURGO Hydrologic Soil Group Data

Slope calculated from areal average within ArcGIS, $0.5\% < s_0 < 64\%$

• Times of concentration estimated using NRCS TR-55 methods:

$$(1000 \text{ cm})^{0.7}$$

$$c = \frac{l}{1140\sqrt{Y}}$$

• Resulting t_c values between 6 min and 3 h, average of 30 min per subbasin • Basin lag times were estimated as 60% of time of concentration, minimum lag time of 3.5 min to ensure model stability

Modeling

30 reaches and 44 junctions identified:

 Muskingum Routing method used to model channel effects • Channels modelled as trapezoidal with bottom width 2', side slope 1:1 and

normal depth of 1 foot to calculate velocity, V

• Travel time through reach approximated as:

$$k = \frac{L}{V}$$

• x was assumed as 0.2 based on other studies

SCS Unit Hydrograph method used to transform rainfall hyetographs to runoff

• Canopy infiltration not significant—most of study area is open field (Figure 4) • Surface infiltration not accounted for due to lack of data

• Highly rural, agricultural area allowed assumption of no significant impervious

• Baseflow not significant– few if any relatively permanent streams, system is comprised almost entirely of wet-weather conveyances

Figure 4: Basin characteristics



• Hydrographs share same timing but differ in maximum flow

Table 2: NOAA Precipitation Frequency Dataset (PFD) Data		
Model	Modelled Flowrate (cfs)	Representative Flow (cfs)
10-year	952	900-1100
50-year	1522	1400-1600
100-year	1792	1700-1900
500-year	2617	2500-2700
Past Event	1549	1400-1600

- changes
- mouth of the cave.
- calibrate the model.
- to forecast flooding based on anticipated rainfall events

- Calculate canopy cover and surface infiltration sections of flow
- discharge rating curve

- Washington, D.C.
- States Depart of Agriculture, Washington, D.C.

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Result: Storm Hydrograph

Conclusions

• Model involves assumptions that must be satisfied for the model to be valid; future work can determine their accuracy or needed

• Higher-resolution elevation data needed, particularly around the

• Flow taken at the mouth of the cave would be beneficial to help

Flood early warning system could be developed from current model

Future Studies

Locate and account for impervious areas within the basin

• Further map the inner dimensions of the cave to determine controlling

• Measure the flow at the entrance of the cave to develop stage-storage-

References

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Acknowledgments